

Chief Executive Officer Michael Thurmond

incomplete and will not be accepted.

DEPARTMENT OF PLANNING & SUSTAINABILITY

Andrew A. Baker, AICP

Director

Application for Certificate of Appropriateness

Date Received: Application No.:		
Address of Subject Property: 1168 Clifton Road, Atlanta, GA 30307		
Applicant: AG Development Group LLC	E-Mail:	aa@mararigroup.com
Applicant Mailing Address:1815 Coventry Road Decatur Georgia 300		
Applicant Phone(s):954-594-4356	Fax:	
Applicant's relationship to the owner: Owner IX Architect: □ Contractor/B		
Owner(s):Anastasiya Arina c/o AG Development Group LLC		
	E-Mail:	
Owner(s) Mailing Address: 1815 Coventry Road, Decatur, GA 30030		
Owner(s) Telephone Number: 954-594-4356 Approximate age or date of construction of the primary structure on the prop project: 1951	erty and any	secondary structures affected by this
Nature of work (check all that apply):		
New construction □ Demolition □ Addition □ Moving a building □ New accessory building □ Landscaping □ Fence/Wall □ Other Sign installation or replacement □ Other □		
Description of Work:		
We are fully renovating interior, but maintain exterior charm of the existing damage, some windows were replaced by the previous owner with vinyl vall exterior double doors with slim steel double doors with clear glass. We match the doors for a cohesive look throughout the house. This house has and heavy forested vegetation. We will also replace the roof with Architec roof over the front balcony/poach if allowed. Replace all exterior decks an finish. We will preserve exterior ceder shingles and replace damaged once exterior charm of the existing house. This form must be completed in its entirety before the Planning Depart supporting documents (plans, material, color samples, photos, etc.). Provide one (1) additional set at scale. All documents submitted in hard copy more than a scale.	vindows. We are also aski s very limited tural Shingles d modify fron the for same. C ment accepts le nine (9) co) collated set	are asking to replace ing to replace all windows with wood windows to visibility from the street due to the elevation s for the main roof and would like to add metal t entrance with an arch entryway with stucco Dur goal is to preserve s it. The form must be accompanied by bilated sets of the application form and all is on paper no larger than 11" x 17" and

application without both the paper and digital forms, or which lacks any of the required attachments, shall be considered

Anastasiya Arina

Signature of Applicant/Date

Revised 8/26/2019



DEPARTMENT OF PLANNING & SUSTAINABILITY

Authorization of a Second Party to Apply for a Certificate of Appropriateness

This form is required if the individual making the request is **<u>not</u>** the owner of the property.

I/We,

being owner(s) of the property at _____

hereby delegate authority to

to file an application for a certificate of appropriateness in my/our behalf.

Signature of Owner(s)

Date

Please review the following information

Approval of this Certificate of Appropriateness does not release the recipient from compliance with all other pertinent county, state, and federal regulations.

Before making any changes to your approved plans, contact the preservation planner (404/371-2155). Some changes may fall within the scope of the existing approval, but others will require review by the preservation commission. If work is performed which is not in accordance with your certificate, a Stop Work Order may be issued.

If your project requires that the county issue a Certificate of Occupancy at the end of construction, an inspection may be made to verify that the work has been completed in accord with the Certificate of Appropriateness. If the work as completed is not the same as that approved in the Certificate of Appropriateness you will not receive a Certificate of Occupancy. You may also be subject to other penalties including fines and/or required demolition of the non-conforming work.

If you do not commence construction within twelve months of the date of approval, your Certificate of Appropriateness will become void and you will need to apply for a new certificate if you still intend to do the work.

APPLICABLE CODES:		FOUNDATION NOT	ES;
INTERNATIONAL BUILDING CODE, 2018 EDITION W/ GA AMENDM	ENTS	-	NG CAPACITIES AS SHOWN ON THE DRAWINGS ARE ESTIMATED, FINA
INTERNATIONAL RESIDENTIAL CODE, 2018 EDITION W. GA AMENDI	MENTS	GEOTECHNICAL EN	VGINEER DURING CONSTRUCTION.
INTERNATIONAL FIRE CODE, 2018 EDITION W/ (NO GA AMENDM			RADE FOR ALL FOOTINGS AND SLABS SHALL BE INSPECTED AND A
INTERNATIONAL PLUMBING CODE, 2018 EDITION W/ GA AMENDA		-	5URE FOR SLAB-ON-GRADE SHALL BE 2,000 PSF, REQUIRED REME
INTERNATIONAL MECHANICAL CODE, 2018 EDITION W/ GA AMEN		GEOTECHNICAL EN	
INTERNATIONAL FUEL GAS CODE, 2018 EDITION W/ GA AMENDM			AND PIERS SHALL REST ON 6" OF 74" CRUSHED STONE BASE MAT
NATIONAL ELECTRICAL CODE, 2020 EDITION (NO GA AMENDME			LE SHOULD BE COMPACTED WITH VIBRATORY PLATE COMPACTOR.
INTERNATIONAL ENERGY CONSERVATION CODE, 2015 EDITION W,	/ GA AMENDMEN13		ED TO PRODUCE CONCRETE SHALL COMPLY WITH THE REQUIREMENT ? OTHERWISE, ALL CONCRETE TO BE NORMAL-WEIGHT AS DEFINED
NFPA IOI LIFE SAFETY CODE 2018 EDITION			SHALL CONTAIN AN APPROVED WATER REDUCING PLASTICIZING AD
FRAMING NOTES:			TAIN AN APPROVED AR-ENTRAINING ADMIXTURE,
I. ALL DIMENSIONS TO BE VERIFIED IN FIELD.			HLORIDE SHALL BE USED IN ANY CONCRETE.
2, CONTRACTOR MUST INFORM ARCHITECT AND ENGINEER OF RECO	RD IF ON-FIELD MEASUREMENTS AND/OR	10, PROVIDE 3" C	OF COVER FOR REINFORCEMENT IN CONCRETE FOOTING.
CONDITIONS VARY FROM DESIGN DRAWINGS.		II. REINFORCING S	TEEL SHALL COMPLY WITH THE REQUIREMENTS OF ASTM A615 AND
3, ALL LUMBER AND PLYWOOD USED FOR FOUNDATION AND/OR I			ING SPLICES SHALL CONFORM TO THE REQUIREMENTS OF ACI 318,
PRESSURE-PRESERVATIVE TREATED AND DRIED AFTER TREATMENT IN SPECIFICATION A, USE CATEGORY 4B AND SECTION 5.2), AND SHA			NS TO BE VERIFIED IN FIELD.
AGENCY.	LE PERIN THE ERVEL OF AN ACCREPTED		AUST BE PROPERLY MARKED PRIOR TO CONSTRUCTION.
4. WHERE LUMBER AND / OR PLYWOOD IS CUT OR DRILLED AFTER T	REATMENT THE TREATED SURFACE SHALL BE		MUST INFORM ARCHITECT AND ENGINEER OF RECORD IF ON-FIELD
FIELD TREATED WITH COPPER NAPHTHENATE, THE CONCENTRATION (-		'S TO CONFORM TO ASTM A307 STANDARDS. 'S TO BE GALVANIZED ACCORDING TO ASTM A153 STANDARDS.
PERCENT COPPER METAL, BY REPEATED BRUSHING, DIPPING OR SOA			STO DE LACVANIZED ACCORDING TO ASTM A199 STANDARDS. ASHER TO BE 11GHTENED AT EACH BOLT.
PRESERVATIVE.			TE SHALL BE EMBEDDED WITH A 1/211 DIAMETER BOLT (WITH NUT
5. ALL TIMBER FRAME CONSTRUCTION SHALL BE DONE IN STRICT CO	ONFORMANCE WITH THE AITC TIMBER		(2) BOLTS PER SILL PLATE SEGMENT WITH ONE BOLT LOCATED I
CONSTRUCTION MANUAL & NDS 2018,			WASHERS SHALL BE PLACED BETWEEN THE FOUNDATION SILL PLACED
6. ALL TIMBER STRUCTURAL FRAMING COMPONENTS, INCLUDING BU		21. ALL BACKFILL	SOILS TO BE COMPACTED TO AT LEAST 95% OF THE MAXIMUM DR
HEADERS, BEAMS, AND POSTS SHALL BE SOUTHERN PINE, #2 OR B			ALL FOUNDATIONS SHALL EXTEND A MINIMUM OF TWELVE (12) IN
7. ALL PLYWOOD FLOOR DECKING SHALL BE 34" APA RATED STRUC			SED TO PRODUCE CONCRETE MASONRY UNITS SHALL COMPLY WITH
8. ALL PLYWOOD ROOF DECKING SHALL BE 5/8" APA RATED STR 9. INTERMEDIATE EXTERIOR PLYWOOD WALL SHEATHING SHALL BE F			ASONRY UNITS SHALL BE NORMAL-WEIGHT, SHALL HAVE NOMINAL
AT 4" O.C. AT PANEL EDGES AND 12" O.C. INTERMEDIATE.	ASTEINED WITH OA CUTWIMUN NAILS DEACED		CMU WALL (FACE EXPOSED TO BACKFILL) SHALL BE WATERPROO
10, ALL LAG BOLT CONNECTIONS SHALL BE PRE-DRILLED WITH THE	PROPER SITE LEAD HOLE DIAMETER IN		PRAINAGE FABRIC SHALL BE WRAPPED AROUND 1211 OF 3/411 CRUS
ACCORDANCE WITH THE AITC TIMBER MANUAL.			HALL BE PLACED AT FULL LENGTH OF THE WALL TO PROVIDE POSIT
II, ALL TIMBER FRAMING CLIPS AND FASTENERS SHALL BE HOT DIP (GALVANIZED AFTER FABRICATION,		? IS RESPONSIBLE ON PROVIDING THE REQUIRED CRAWL SPACE VEN MASONRY CONSTRUCTION SHALL BE TYPE S COMPLYING WITH THE
12. ALL FRAMING CONNECTORS FOR STRUCTURAL TIMBER MEMBERS	SHALL BE SIMPSON STRONG TIE		EA COMPRESSIVE STRENGTH OF MASONRY (f'm) SHALL BE 2,000
CONNECTORS AND SHALL HAVE A MINIMUM CAPACITY OF 1100 LBS	- 2.		NFORCEMENT, TIES, AND OTHER ACCESSORIES SHALL BE RESISTAN
13. CONTRACTOR MUST ABIDE BY ALL ENGINEERED LUMBER MANUE/	ACTURER, OPEN WEB TRUSS MANUFACTURER,		D BED JOINTS SHALL BE $\frac{2}{3}$ " THICK, BED JOINTS OF THE STARTING C
AND FRAMING CONNECTOR MANUFACTURER RECOMMENDATIONS A			DEEP BY 3.5" WDE CONTINUOUS KEYWAY
14. SILL PLATES AND SOLE PLATES SHALL BE PROTECTED AGAINST			ASONRY SHALL COMPLY WITH THE REQUIREMENTS OF ASTM C476
15. EACH SILL PLATE SHALL BE EMBEDDED WITH A 1/2" DIAMETER			55 OR LADDER TYPE HORIZONTAL JOINT REINFORCEMENT COMPLY
MORE THAN 72 INCHES ON CENTER AND EMBEDDED AT LEAST 7 IN		OTHERWISE, PROV	/IDE PREFABRICATED ''LO 5'' AT WALL CORNERS AND ''TO 5'' AT W
MINIMUM OF TWO (2) BOLTS PER SILL PLATE SEGMENT WITH ONE SEGMENT AT LEAST 4 INCHES BUT NOT MORE THAN 12 INCHES	BOLT LOCATED FROM THE END OF EACH	36, LAY ALL CON	ICRETE MASONRY UNITS IN RUNNING BOND.
16, ALL LVL BEAMS 10 BE BOISE CASCADE VERSA LAM 2, 12 3100	Fb		
17. HOLD-DOWNS MUST BE PROVIDED ON EITHER SIDE OF ALL LARC			RESIDENTIA
(AT THE NARROW-BRACED WALL PANELS), AT SUCH OPENINGS SIN			
AT THE FLOOR PLATE AND STRAPS AT THE TOP ARE REQUIRED.			
			1168 CL
	ROOF FRAMING NOTES;) ATLANTA
DESIGN CRITERIA LIVE LOADS:	I, COLLAR TIES SHALL BE LOCATED WITHIN IE	3" OF BOTTOM	AILANIA
<u>LIVE LOADS:</u> FL <i>OO</i> R (NON SLEEPING AREAS) - 40 PSF	OF RIDGE BOARD		DEKAL
TEOUR (NON JEEFING AREAD) = 90 F J	2. COLLAR THE SHALL BE USED TO CONNECT		
EL OOP (SLEEPING APEAS) - 30 PSE	Z, CULLAK TE PHALL VE UPEN TO CONNECT	OPPOSING	
FLOOR (SLEEPING AREAS) - 30 PSF (EILING - 20 PSF	RAFTER	OPPOSING	GENERAL SCOPE OF WORK NOTES: L REDESIGN INTERIOR LAYOUT AND TO ACHEVE A
FL <i>OO</i> R (SLEEPING AREAS) - 30 PSF CEILING - 20 PSF R <i>OO</i> F - 20 PSF	RAFTER 3, RAFTERS SHALL BE FRAMED NOT MORE TH	IAN 1-1/2''	I. REDESIGN INTERIOR LAYOUT AND TO ACHIEVE A BEDROOMS AND BATHROOMS.
CEILING - 20 PSF	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE 11 OFFSET FROM EACH OTHER TO THE RIDGE BC	1AN 1-1/2'' 7ARD	I. REDESIGN INTERIOR LAYOUT AND TO ACHEVE AI BEDROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & FLOOR FRAMING HOUSE THAT WILL HAVE A 2ND LEVEL.
CEILING - 20 PSF ROOF - 20 PSF DEAD LOADS:	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE TH OFFSET FROM EACH OTHER TO THE RIDGE BC 4. WHERE CEILING JOISTS ARE NOT CONNEC	IAN I-1/2'' DARD TED TO RAFTERS	I. REDESIGN INTERIOR LAVOUT AND TO ACHEVE AI BEDROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & FLOOR FRAMING HOUSE THAT WILL HAVE A 2ND LEVEL. 3. CONSTRUCT A NEW DORMER ON 2ND LEVEL TI BATHROOM.
CEILING - 20 PSF ROOF - 20 PSF <u>DEAD LOADS:</u> FLOOR - 10 PSF	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE TH OFFSET FROM EACH OTHER TO THE RIDGE BC 4. WHERE CEILING JOISTS ARE NOT CONNEC AT TOP WALL PLATE, A 2X4 CONTINUOUS RA	IAN I-1/2'' DARD TED TO RAFTERS FTER TIE MUST BE	I. REDESIGN INTERIOR LAYOUT AND TO ACHEVE A BEDROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & FLOOR FRAMIN HOUSE THAT WILL HAVE A 2ND LEVEL. 3. CONSTRUCT A NEW DORMER ON 2ND LEVEL T BATHROOM. 4. REDESIGN STAIRCASE TO 2ND LEVEL.
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CEILING - 20 PSF ROOF - 20 PSF <u>DEAD LOADS:</u> FLOOR - 10 PSF	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE TH OFFSET FROM EACH OTHER TO THE RIDGE BC 4. WHERE CEILING JOISTS ARE NOT CONNEC AT TOP WALL PLATE, A 2X4 CONTINUOUS RA	IAN I-1/2'' DARD TED TO RAFTERS FTER TIE MUST BE	I. REDESIGN INTERIOR LAYOUT AND TO ACHEVE A BEPROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & FLOOR FRAMIN HOUSE THAT MULL HAVE A 2ND LEVEL. 3. CONSTRUCT A NEW DORMER ON 2ND LEVEL T BATHROOM. 4. REDESIGN STAIRCASE TO 2ND LEVEL. DRAWING NO. COVER SHEET
CEILING - 20 PSF ROOF - 20 PSF <u>DEAD LOADS:</u> FLOOR - 10 PSF CEILING - 10 PSF ROOF - 10 PSF	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE TH OFFSET FROM EACH OTHER TO THE RIDGE BC 4. WHERE CEILING JOISTS ARE NOT CONNEC AT TOP WALL PLATE, A 2X4 CONTINUOUS RA	IAN I-1/2'' DARD TED TO RAFTERS FTER TIE MUST BE	I. REDESIGN INTERIOR LAYOUT AND TO ACHEVE A BEDROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & FLOOR FRAMIN HOUSE THAT WILL HAVE A 2ND LEVEL. 3. CONSTRUCT A NEW DORMER ON 2ND LEVEL T BATHROOM. 4. REDESIGN STARCASE TO 2ND LEVEL. DRAWING NO. COVER SHEET 5T-1 FLOOR, CEILING, ROOF RAFTER & I-JO
CEILING - 20 PSF ROOF - 20 PSF <u>DEAD LOADS:</u> FLOOR - 10 PSF CEILING - 10 PSF ROOF - 10 PSF <u>WIND DATA:</u>	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE TH OFFSET FROM EACH OTHER TO THE RIDGE BC 4. WHERE CEILING JOISTS ARE NOT CONNEC AT TOP WALL PLATE, A 2X4 CONTINUOUS RA	IAN I-1/2'' DARD TED TO RAFTERS FTER TIE MUST BE	I. REDESIGN INTERIOR LAVOUT AND TO ACHEVE AN BEDROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & FLOOR FRAMING HOUSE THAT WILL HAVE A 2ND LEVEL. 3. CONSTRUCT A NEW PORMER ON 2ND LEVEL TO BATHROOM. 4. REDESIGN STAIRCASE TO 2ND LEVEL. DRAWING NO. COVER SHEET ST-1 FLOOR, CEILING, ROOF RAFTER & I-JC EC-1 MAIN FLOOR EXISTING CONDITION & D
CEILING - 20 PSF ROOF - 20 PSF <u>PEAD LOADS:</u> FLOOR - 10 PSF CEILING - 10 PSF ROOF - 10 PSF <u>WIND DATA:</u> BASIC WIND SPEED (3 SEC GUST) - 106 MPH	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE TH OFFSET FROM EACH OTHER TO THE RIDGE BC 4. WHERE CEILING JOISTS ARE NOT CONNEC AT TOP WALL PLATE, A 2X4 CONTINUOUS RA	IAN I-1/2'' DARD TED TO RAFTERS FTER TIE MUST BE	I. REDESIGN INTERIOR LAYOUT AND TO ACHEVE A BEPROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & FLOOR FRAMIN HOUSE THAT WILL HAVE A 2ND LEVEL. 3. CONSTRUCT A NEW DORMER ON 2ND LEVEL T BATHROOM. 4. REDESIGN STARCASE TO 2ND LEVEL. DRAWING NO. COVER SHEET ST-1 FLOOR, CEILING, ROOF RAFTER & I-JC EC-1 MAIN FLOOR EXISTING CONDITION & D EC-2 2ND FLOOR EXISTING CONDITION & D
CEILING - 20 PSF ROOF - 20 PSF <u>DEAD LOADS:</u> FLOOR - 10 PSF CEILING - 10 PSF ROOF - 10 PSF <u>WIND DATA:</u> BASIC WIND SPEED (3 SEC GUST) - 106 MPH RISK CATEGORY - CATEGORY II	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE TH OFFSET FROM EACH OTHER TO THE RIDGE BC 4. WHERE CEILING JOISTS ARE NOT CONNEC AT TOP WALL PLATE, A 2X4 CONTINUOUS RA	IAN I-1/2'' DARD TED TO RAFTERS FTER TIE MUST BE	I. REDESIGN INTERIOR LAVOUT AND TO ACHEVE AN BEDROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & FLOOR FRAMING HOUSE THAT WILL HAVE A 2ND LEVEL. 3. CONSTRUCT A NEW DORMER ON 2ND LEVEL TO BATHROOM. 4. REDESIGN STAIRCASE TO 2ND LEVEL. DRAWING NO.
CEILING - 20 PSF ROOF - 20 PSF <u>PEAD LOADS:</u> FLOOR - 10 PSF CEILING - 10 PSF ROOF - 10 PSF <u>WIND DATA:</u> BASIC WIND SPEED (3 SEC GUST) - 106 MPH	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE TH OFFSET FROM EACH OTHER TO THE RIDGE BC 4. WHERE CEILING JOISTS ARE NOT CONNEC AT TOP WALL PLATE, A 2X4 CONTINUOUS RA	IAN I-1/2'' DARD TED TO RAFTERS FTER TIE MUST BE	I. REDESIGN INTERIOR LAYOUT AND TO ACHEVE A DEDROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & PLOOR PRAMIN HOUSE THAT WILL HAVE A 2ND LEVEL. 3. CONSTRUCT A NEW DORMER ON 2ND LEVEL T DATAROOM. 4. REDESIGN STARCASE TO 2ND LEVEL. DRAWING NO. COVER SHEET 51-1 FLOOR, CEILING, ROOF RAFTER & I-JC EC-1 MAIN FLOOR EXISTING CONDITION & D A-1 MAIN FLOOR PLAN, DOOR & WINDOW A-1A 2ND LEVEL FLOOR PLAN, DOOR & WINDOW
CEILING - 20 PSF ROOF - 20 PSF <u>PEAD LOADS:</u> FLOOR - 10 PSF CEILING - 10 PSF ROOF - 10 PSF <u>WIND DATA:</u> BASIC WIND SPEED (3 SEC GUST) - 106 MPH RISK CATEGORY - CATEGORY II WIND EXPOSURE - EXPOSURE B	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE TH OFFSET FROM EACH OTHER TO THE RIDGE BC 4. WHERE CEILING JOISTS ARE NOT CONNEC AT TOP WALL PLATE, A 2X4 CONTINUOUS RA	IAN I-1/2'' DARD TED TO RAFTERS FTER TIE MUST BE	I. REDESIGN INTERIOR LAVOUT AND TO ACHEVE AN DEDROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & FLOOR FRAMING HOUSE THAT WILL HAVE A 2ND LEVEL. 3. CONSTRUCT A NEW DORNER ON 2ND LEVEL TO BATHROOM. 4. REDESIGN STAIRCASE TO 2ND LEVEL. DRAWING NO. DRAWING NO. DRAWING NO. DRAWING NO. DRAWING NO. DRAWING NO. COVER SHEET ST-1 FLOOR, CEILING, ROOF RAFTER & I-JC EC-1 MAIN FLOOR EXISTING CONDITION & D EC-2 2ND FLOOR EXISTING CONDITION & D A-1 MAIN FLOOR PLAN, DOOR & WINDOW A-1A 2ND LEVEL FLOOR PLAN, DOOR & WI
CEILING - 20 PSF ROOF - 20 PSF <u>PEAD LOADS:</u> FLOOR - 10 PSF CEILING - 10 PSF ROOF - 10 PSF <u>WIND DATA:</u> BASIC WIND SPEED (3 SEC GUST) - 106 MPH RISK CATEGORY - CATEGORY II	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE TH OFFSET FROM EACH OTHER TO THE RIDGE BC 4. WHERE CEILING JOISTS ARE NOT CONNEC AT TOP WALL PLATE, A 2X4 CONTINUOUS RA	IAN I-1/2'' DARD TED TO RAFTERS FTER TIE MUST BE	I. REDESIGN INTERIOR LAYOUT AND TO ACHIEVE AN BEDROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & FLOOR FRAMING HOUSE THAT WILL HAVE A 2ND LEVEL. 3. CONSTRUCT A NEW DORWER ON 2ND LEVEL TO BATHROOM. 4. REDESIGN STARCASE TO 2ND LEVEL. DRAWING NO. COVER SHEET ST-1 FLOOR, CEILING, ROOF RAFTER & I-JC EC-1 MAIN FLOOR EXISTING CONDITION & D EC-2 2ND FLOOR EXISTING CONDITION & D A-1 MAIN FLOOR PLAN, DOOR & WINDOW A-1A 2ND LEVEL FLOOR PLAN, DOOR & WI A-2 NEW EXTERIOR ELEVATIONS A-3 NEW FOUNDATION/FLOOR FRAMING,
CEILING - 20 PSF ROOF - 20 PSF <u>PEAD LOADS:</u> FLOOR - 10 PSF CEILING - 10 PSF ROOF - 10 PSF <u>WIND DATA:</u> BASIC WIND SPEED (3 SEC AUST) - 106 MPH RISK CATEGORY - CATEGORY II WIND EXPOSURE - EXPOSURE B	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE TH OFFSET FROM EACH OTHER TO THE RIDGE BC 4. WHERE CEILING JOISTS ARE NOT CONNEC AT TOP WALL PLATE, A 2X4 CONTINUOUS RA	IAN I-1/2'' DARD TED TO RAFTERS FTER TIE MUST BE	I. REDESIGN INTERIOR LAYOUT AND TO ACHEVE A BEDROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & FLOOR FRAMIN HOUSE THAT WILL HAVE A 2ND LEVEL. 3. CONSTRUCT A NEW DORMER ON 2ND LEVEL T BATHROOM. 4. REDESIGN STARCASE TO 2ND LEVEL. DRAWING NO. COVER SHEET ST-1 FLOOR, CEILING, ROOF RAFTER & I-JC EC-1 MAIN FLOOR EXISTING CONDITION & D EC-2 2ND FLOOR EXISTING CONDITION & D A-1 MAIN FLOOR PLAN, DOOR & WINDOW A-1A 2ND LEVEL FLOOR PLAN, DOOR & WI A-2 NEW EXTERIOR ELEVATIONS A-5 NEW FOUNDATION/FLOOR FRAMING, A-4 NEW DORMER ADDITION ROOF FRAMING
CEILING - 20 PSF ROOF - 20 PSF <u>PEAD LOADS:</u> FLOOR - 10 PSF CEILING - 10 PSF ROOF - 10 PSF <u>WIND DATA:</u> BASIC WIND SPEED (3 SEC AUST) - 106 MPH RISK CATEGORY - CATEGORY II WIND EXPOSURE - EXPOSURE B <u>DESIGN BEARING PRESSURE:</u> 2000 PSF	RAFTER 3. RAFTERS SHALL BE FRAMED NOT MORE TH OFFSET FROM EACH OTHER TO THE RIDGE BC 4. WHERE CEILING JOISTS ARE NOT CONNEC AT TOP WALL PLATE, A 2X4 CONTINUOUS RA	IAN I-1/2'' DARD TED TO RAFTERS FTER TIE MUST BE	I. REDESIGN INTERIOR LAYOUT AND TO ACHIEVE AN BEDROOMS AND BATHROOMS. 2. RECONSTRUCT FOUNDATION & FLOOR FRAMING HOUSE THAT WILL HAVE A 2ND LEVEL. 3. CONSTRUCT A NEW DORMER ON 2ND LEVEL TO BATHROOM. 4. REDESIGN STAIRCASE TO 2ND LEVEL. DRAWING NO. BEC-1 MAIN FLOOR EXISTING CONDITION & D A-1 MAIN FLOOR PLAN, DOOR & WIN

INAL, EXACT SOIL BEARING CAPACITIES SHALL BE FIELD DETERMINED AND VERIFIED BY THE OWNER'S SOIL TESTING LABORATORY AND/OR

APPROVED BY THE OWNER'S TESTING LABORATORY IMMEDIATELY PRIOR TO PLACING FOUNDATION CONCRETE. MEDIATION TO ACHIEVE DESIGN BEARING PRESSURE SHALL BE APPLIED. MEANS AND METHODS OF REMEDIATION SHALL COME FROM LICENSED

ATERIAL, ALL CRUSHED STONE SHOULD COMPLY WITH ASTM D2940.

ENTS OF ACI 318-14. ED BY ACI 318-14 AND ITS 28-DAY CONCRETE COMPRESSIVE STRENGTH SHALL BE 3,500 PSI.

ADMIXTURE, APPROVED HIGH-RANGE WATER REDUCING ADMIXTURES MAY BE UTILIZED, ALL CONCRETE PERMANENTLY EXPOSED TO WEATHER

ND SHALL HAVE MINIMUM YIELD STRENGTH OF 60,000 PSI. 18, BLIT IN NO CASE SHALL BE LESS THAN 36 BAR DIAMETERS, UNLESS NOTED OTHERWISE.

D MEASUREMENTS AND / OR CONDITIONS VARY FROM DESIGN DRAWING.

IUT AND WASHER) SPACED NOT MORE THAN 72 INCHES ON CENTER AND EMBEDDED AT LEAST 7 INCHES INTO CONCRETE. THERE SHALL BE A ED FROM THE END OF EACH SEGMENT AT LEAST 4 INCHES BUT NOT MORE THAN 12 INCHES.

LATE AND THE NUT. SUCH WASHERS SHALL BE A MINIMUM OF 0.229 INCH BY 3 INCHES BY 3 INCHES

DRY DENSITY, AS DETERMINED BY STANDARD PROCTOR TEST (ASTM D698).

INCHES BELOW THE TOP OF FINISHED GRADE.

17H THE REQUIREMENTS OF TMS 402/602-16.

AL DIMENSIONS OF 8''X8''X16'', AND SHALL COMPLY WITH THE REQUIREMENTS OF ASTM C90 WITH A MAXIMUM DENSITY OF 105 PCF.

ROOFED WITH TWO CONTINUOUS COATINGS OF HOT BITUMINOUS MATERIAL

RUSHED STONE

SITIVE DRAINAGE

'ENTILATION AREA, SPECS, AND MATERIALS CONFORMING WITH GEORGIA IRC 2018 R408

HE REQUIREMENTS OF ASTM C270 WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 1,800 PSI

OO P51, ANT T*O CORROSIO*N

G COURSE OVER THE CONCRETE FOUNDATION MAY BE BETWEEN 4"10 74"

76 AND SHALL HAVE MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,000 PSI

"LYING WITH ASTM A951 WITH MINIMUM TWO 9 GA LONGITUDINAL LINES, ZINC COATED, PLACED 16 INCHES ON CENTER UNLESS NOTED I WALL INTERSECTIONS

AL RENOVATION ELIFTON ROAD A, GA 30307 AB COUNTY	LB Designs ARCHITECTURAL SERVICES COLLEGE PARK, GA (404) 421-3272
AWING ON THE RIGHT SIDE OF THE VEL TO ENCOMPASS NEW MASTER RESIDENCE S.F. CALCILATIONS MAIN FLOOR - 2,625 S.F. HEATED SPACE 2ND FLOOR - 885 S.F. HEATED SPACE AWING DESCRIPTION I-JOISTS SPAN TABLES I & DEMOLITION PLANS	$\begin{array}{c} F O R G \\ G F G S TERED \\ \star S - 5 2022 \\ \star No. PE043515 \\ PROFESSIONAL \end{array}$
& DEMOLITION PLANS X POW SCHEDULES, INT. ELEVATIONS AND GENERAL NOTES X & WINDOW SCHEDULES, INT. ELEVATIONS AND GENERAL NOTES X ING, 2ND FLOOR FRAMING & CEILING FRAMING PLANS & DETAILS X	A CONCINEER INT
AMING & OVERALL ROOF PLAN	

ISSUED FOR CONSTRUCTION

Southern Pine Span Tables

Maximum spans given in feet and inches Inside to inside of bearings

Table 2	? Floor J	oists –	40 PSF l	IVE LOAL	о, 10 рs	F DEAD L	oad, 36	O DEFLEC	TION				
Size	Spacing	Grade											
inches	inches on center	Visually Graded				Machin	e Stress Rate	ed (MSR)	Machine E	valuated Lu	nber (MEL		
		DSS	No.1	No.2	No.3	2400f - 2.0E	1650f - 1.5E	1500f - 1.6E	M-14 (1800-1.7)	M-29 (1550-1.7)	M-12 (1600-1		
	12.0	11-4	10-9	10-3	8-2	11-7	10-6	10-9	10-11	10-11	10-9		
2x6	16.0	10-4	9-9	9-4	7-1	10-6	9-6	9-9	9-11	9-11	9-9		
280	19.2	9-8	9-2	8-6	6-5	9-10	9-0	9-2	9-4	9-4	9-2		
	24.0	9-0	8-6	7-7	5-9	9-2	8-4	8-6	8-8	8-8	8-6		
	12.0	15-0	14-2	13-6	10-3	15-3	13-10	14-2	14-5	14-5	14-2		
2x8	16.0	13-7	12-10	11 - 10	8-11	13-10	12-7	12-10	13-1	13-1	12-10		
210	19.2	12-10	12-1	10-10	8-2	13-0	11 - 10	12-1	12-4	12-4	12-1		
	24.0	11 - 11	11-3	9-8	7-3	12-1	11-0	11-3	11-5	11-5	11-3		
	12.0	19-1	18-0	16-2	12-6	19-5	17-8	18-0	18-5	18-5	18-0		
2x10	16.0	17-4	16-1	14-0	10-10	17-8	16-0	16-5	16-9	16-9	16-5		
2210	19.2	16-4	14-8	12-10	9-10	16-7	15-1	15-5	15-9	15-9	15-5		
	24.0	15-2	13-1	11-5	8-10	15-5	14-0	14-4	14-7	14-7	14-4		
	12.0	23-3	21-11	19-1	14-9	23-7	21-6	21-11	22-5	22-5	21-11		
2x12	16.0	21-1	19-1	16-6	12-10	21-6	19-6	19-11	20-4	20-4	19-11		
2812	19.2	19-10	17-5	15-1	11-8	20-2	18-4	18-9	19-2	19-2	18-9		
	24.0	18-5	15-7	13-6	10-5	18-9	17-0	17-5	17-9	17-9	17-5		

The spans in these tables were determined on the same basis as the code-recognized *Span Tables for Joists & Rafters* and *Wood Structural Design Data*, both published by the American Wood Council; concentrated loads and uplift loads caused by wind were not considered. See *Using These Tables* and *Design Assumptions* for additional information. Applied loads are given in pounds per square foot (psf). Deflection is limited to the span in inches divided by 360, 240 or 180 and is based on live load only. The load duration factor, C_D, is 1.0 unless shown as 1.15 for snow or 1.25 for construction loads. Listed spans are for dry-service conditions unless the table is labeled as Wet-Service. Check sources of supply for available grades and sizes, and for lumber longer than 20 feet; an asterisk (*) indicates the listed span has been limited to 26'-0" based on availability.

Reference design values for Southern Pine lumber are published by the Southern Pine Inspection Bureau after approval by the Board of Review of the American Lumber Standard Committee. Reference design values are based on normal load duration under the moisture service conditions specified. Because the strength of wood varies with conditions under which it is used, design values should only be applied in conjunction with appropriate design and service recommendations from the National Design Specification[®] (NDS[®]) for Wood Construction published by the American Wood Council. The Southern Forest Products Association (SFPA) does not test lumber or establish design values. Accordingly, neither SFPA, nor its members, warrant that the design values and adjustment factors on which the span tables are based are correct, and disclaim responsibility for injury or damage resulting from the use of such span tables.

The conditions under which lumber is used in construction may vary widely, as does the quality of workmanship. Neither SFPA, nor its members, have knowledge of the quality of the materials, workmanship or construction methods used on any construction project, and, accordingly, do not warrant the technical data, design or performance of the lumber in completed structures.

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MAXIMUM SPANS: SOUTHERN PINE JOISTS & RAFTERS SOUTHERN FOREST PRODUCTS ASSOCIATION

Southern Pine Span Tables

Maximum spans given in feet and inches Inside to inside of bearings

TABLE 16 Ceiling Joists 20 psf Live Load, 10 psf Dead Load, 240 Deflection

Size	Spacing	Grade											
inches	inches on center	Visually Graded				Machine Stress Rated (MSR)			Machine Evaluated Lumber (MEL)				
	on contor	DSS	No.1	No.2	No.3	2400f - 2.0E	1650f - 1.5E	1500f - 1.6E	M-14 (1800-1.7)	M-29 (1550-1.7)	M-12 (1600-1.6)		
	12.0	10-5	9-10	9-3	7-2	10-7	9-8	9-10	10-0	10-0	9-10		
2x4	16.0	9-6	8-11	8-0	6-2	9-8	8-9	8-11	9-1	9-1	8-11		
284	19.2	8-11	8-5	7-4	5-8	9-1	8-3	8-5	8-7	8-7	8-5		
	24.0	8-3	7-8	6-7	5-1	8-5	7-8	7-8	8-0	7-9	7-10		
	12.0	16-4	15-6	13-11	10-7	16-8	15-2	15-6	15-9	15-9	15-6		
2x6	16.0	14-11	14-0	12-0	9-2	15-2	13-9	14-1	14-4	14-4	14-1		
280	19.2	14-0	12-9	11-0	8-4	14-3	12-11	13-3	13-6	13-6	13-3		
	24.0	13-0	11-5	9-10	7-5	13-3	12-0	12-0	12-6	12-3	12-3		
	12.0	21-7	20-5	17-7	13-3	21-11	19-11	20-5	20-10	20-10	20-5		
2x8	16.0	19-7	17-9	15-3	11-6	19-11	18-2	18-6	18-11	18-11	18-6		
280	19.2	18-5	16-2	13-11	10-6	18-9	17-1	17-5	17-9	17-9	17-5		
	24.0	17-2	14-6	12-6	9-5	17-5	15-10	15-10	16-6	16-2	16-2		
	12.0	26-0*	23-11	20-11	16-1	26-0*	25-5	26-0	26-0*	26-0*	26-0		
2x10	16.0	25-0	20-9	18-1	13-11	25-5	23-2	23-8	24-1	24-1	23-8		
2X IU	19.2	23-7	18-11	16-6	12-9	23-11	21-9	22-3	22-8	22-8	22-3		
	24.0	21-10	16-11	14-9	11-5	22-3	20-2	20-3	21-1	20-7	20-8		

The spans in these tables were determined on the same basis as the code-recognized *Span Tables for Joists & Rafters* and *Wood Structural Design Data*, both published by the American Wood Council; concentrated loads and uplift loads caused by wind were not considered. See *Using These Tables* and *Design Assumptions* for additional information. Applied loads are given in pounds per square foot (psf). Deflection is limited to the span in inches divided by 360, 240 or 180 and is based on live load only. The load duration factor, C_D, is 1.0 unless shown as 1.15 for snow or 1.25 for construction loads. Listed spans are for dry-service conditions unless the table is labeled as Wet-Service. Check sources of supply for available grades and sizes, and for lumber longer than 20 feet; an asterisk (*) indicates the listed span has been limited to 26'-0" based on availability.

Reference design values for Southern Pine lumber are published by the Southern Pine Inspection Bureau after approval by the Board of Review of the American Lumber Standard Committee. Reference design values are based on normal load duration under the moisture service conditions specified. Because the strength of wood varies with conditions under which it is used, design values should only be applied in conjunction with appropriate design and service recommendations from the National Design Specification[®] (NDS[®]) for Wood Construction published by the American Wood Council. The Southern Forest Products Association (SFPA) does not test lumber or establish design values. Accordingly, neither SFPA, nor its members, warrant that the design values and adjustment factors on which the span tables are based are correct, and disclaim responsibility for injury or damage resulting from the use of such span tables.

The conditions under which lumber is used in construction may vary widely, as does the quality of workmanship. Neither SFPA, nor its members, have knowledge of the quality of the materials, workmanship or construction methods used on any construction project, and, accordingly, do not warrant the technical data, design or performance of the lumber in completed structures.



MAXIMUM SPANS: SOUTHERN PINE JOISTS & RAFTERS SOUTHERN FOREST PRODUCTS ASSOCIATION Copyright © 2013. Southern Forest Products Association. All rights reserved.

L/480 Live Load Deflection

Depth TJI® 40 PSF Live Load / 10 PSF Dead Load 40 PSF Live Load 12" o.c. 16" o.c. 19.2" o.c. 24" o.c. 12" o.c. 16" o.c. 110 16'-11" 15'-6" 14'-7" 13'-7" 16'-11" 15'-6" 9½" 210 17'-9" 16'-3" 15'-4" 14'-3" 17'-9" 16'-3"							F Live Load	20 PSF Dea	d L
vepth	IN®	12" o.c.	16" o.c.	19.2" o.c.	24" o.c.	12" o.c.	16" o.c.	19.2" o.c.	
	110	16'-11"	15'-6"	14'-7"	13'-7"	16'-11"	15'-6"	14'-3"	
91/2"	210	17'-9"	16'-3"	15'-4"	14'-3"	17'-9"	16'-3"	15'-4"	
	230	18'-3"	16'-8"	15'-9"	14'-8"	18'-3"	16'-8"	15'-9"	
	110	20'-2"	18'-5"	17'-4"	15'-9"(1)	20'-2"	17'-8"	16'-1"(1)	
	210	21'-1"	19'-3"	18'-2"	16'-11"	21'-1"	19'-3"	17'-8"	
111/8"	230	21'-8"	19'-10"	18'-8"	17'-5"	21'-8"	19'-10"	18'-7"	
	360	22'-11"	20'-11"	19'-8"	18'-4"	22'-11"	20'-11"	19'-8"	1
	560	26'-1"	23'-8"	22'-4"	20'-9"	26'-1"	23'-8"	22'-4"	
	110	22'-10"	20'-11"	19'-2"	17'-2"(1)	22'-2"	19'-2"	17'-6"(1)	
	210	23'-11"	21'-10"	20'-8"	18'-10"(1)	23'-11"	21'-1"	19'-2"(1)	
14"	230	24'-8"	22'-6"	21'-2"	19'-9"(1)	24'-8"	22'-2"	20'-3"(1)	
	360	26'-0"	23'-8"	22'-4"	20'-9"(1)	26'-0"	23'-8"	22'-4" ⁽¹⁾	1
	560	29'-6"	26'-10"	25'-4"	23'-6"	29'-6"	26'-10"	25'-4"(1)	2
	110	25'-4"	22'-6"	20'-7"(1)	18'-1"(1)	23'-9"	20'-7"(1)	18'-9"(1)	
	210	26'-6"	24'-3"	22'-6"(1)	19'-11"(1)	26'-0"	22'-6"(1)	20'-7"(1)	
16"	230	27'-3"	24'-10"	23'-6"	21'-1"(1)	27'-3"	23'-9"	21'-8"(1)	
	360	28'-9"	26'-3"	24'-8"(1)	21'-5"(1)	28'-9"	26'-3"(1)	22'-4"(1)	1
	560	32'-8"	29'-8"	28'-0"	25'-2"(1)	32'-8"	29'-8"	26'-3"(1)	2

Southern Pine Span Tables

Maximum spans given in feet and inches Inside to inside of bearings

Size	Spacing	Grade											
inches	inches on center		Visually	Graded		Machin	e Stress Rate	ed (MSR)	Machine E	evaluated Lu	nber (MEL)		
		DSS	No.1	No.2	No.3	2400f - 2.0E	1650f - 1.5E	1500f - 1.6E	M-14 (1800-1.7)	M-29 (1550-1.7)	M-12 (1600-1.		
	12.0	13-0	12-3	11-7	8-9	13-3	12-0	12-3	12-6	12-6	12-3		
2x6	16.0	11 - 10	11-2	10-0	7-7	12-0	10-11	11-2	11-5	11-5	11-2		
280	19.2	11-1	10-6	9-2	6-11	11-4	10-3	10-6	10-8	10-8	10-6		
	24.0	10-4	9-6	8-2	6-2	10-6	9-6	9-9	9-11	9-11	9-9		
	12.0	17-2	16-2	14-8	11-0	17-5	15-10	16-2	16-6	16-6	16-2		
2x8	16.0	15-7	14-8	12-8	9-7	15-10	14-5	14-8	15-0	15-0	14-8		
220	19.2	14-8	13-5	11-7	8-9	14-11	13-6	13-10	14-1	14-1	13-10		
	24.0	13-7	12-0	10-4	7-10	13-10	12-7	12-10	13-1	13-1	12-10		
	12.0	21-10	19-11	17-4	13-5	22-3	20-2	20-8	21-1	21-1	20-8		
2x10	16.0	19-10	17-3	15-1	11-7	20-2	18-4	18-9	19-2	19-2	18-9		
2210	19.2	18-8	15-9	13-9	10-7	19-0	17-3	17-8	18-0	18-0	17-8		
	24.0	17-4	14-1	12-3	9-6	17-8	16-0	16-5	16-9	16-9	16-5		
	12.0	26-0*	23-7	20-5	15-10	26-0*	24-7	25-1	25-7	25-7	25-1		
2x12	16.0	24-2	20-5	17-9	13-9	24-7	22-4	22-10	23-3	23-3	22-10		
2312	19.2	22-9	18-8	16-2	12-6	23-1	21-0	21-6	21-11	21 - 11	21-6		
	24.0	21-1	16-8	14-6	11-2	21-6	19-6	19-11	20-4	20-4	19-11		

The spans in these tables were determined on the same basis as the code-recognized *Span Tables for Joists & Rafters* and *Wood Structural Design Data*, both published by the American Wood Council; concentrated loads and uplift loads caused by wind were not considered. See *Using These Tables* and *Design Assumptions* for additional information. Applied loads are given in pounds per square foot (psf). Deflection is limited to the span in inches divided by 360, 240 or 180 and is based on live load only. The load duration factor, C_D, is 1.0 unless shown as 1.15 for snow or 1.25 for construction loads. Listed spans are for dry-service conditions unless the table is labeled as Wet-Service. Check sources of supply for available grades and sizes, and for lumber longer than 20 feet; an asterisk (*) indicates the listed span has been limited to 26⁻⁰" based on reliability.

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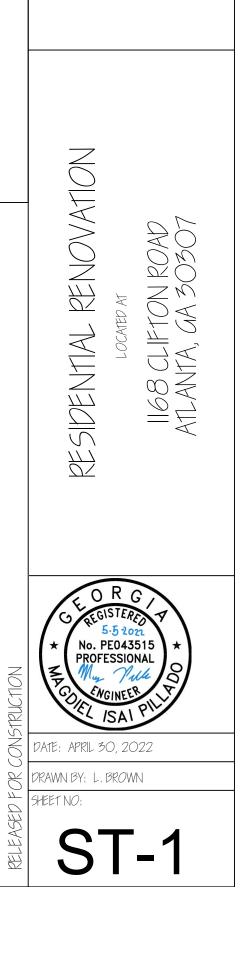
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NEW DESIGN VALUES 2013 Edition

MAXIMUM SPANS: SOUTHERN PINE JOISTS & RAFTERS SOUTHERN FOREST PRODUCTS ASSOCIATION

Load 24" o.c. 12'-9" 14'-0" 14'-8" 14'-4"(1) 15'-9"(1) 16'-7"(1) 17'-10"(1) 20'-9"(1) 15'-0"(1) 16'-7"(1) 17'-6"(1) 17'-10"(1) 20'-11"(1) 15'-0"(1) 16'-7"(1) 17'-6"(1) 17'-10"(1) 20'-11"(1)



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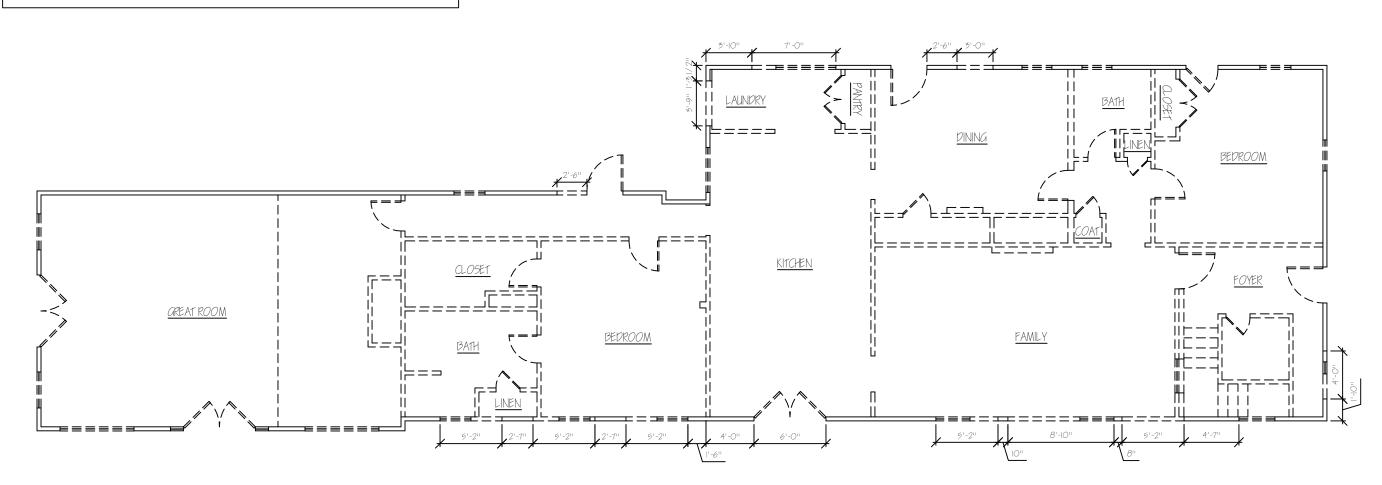
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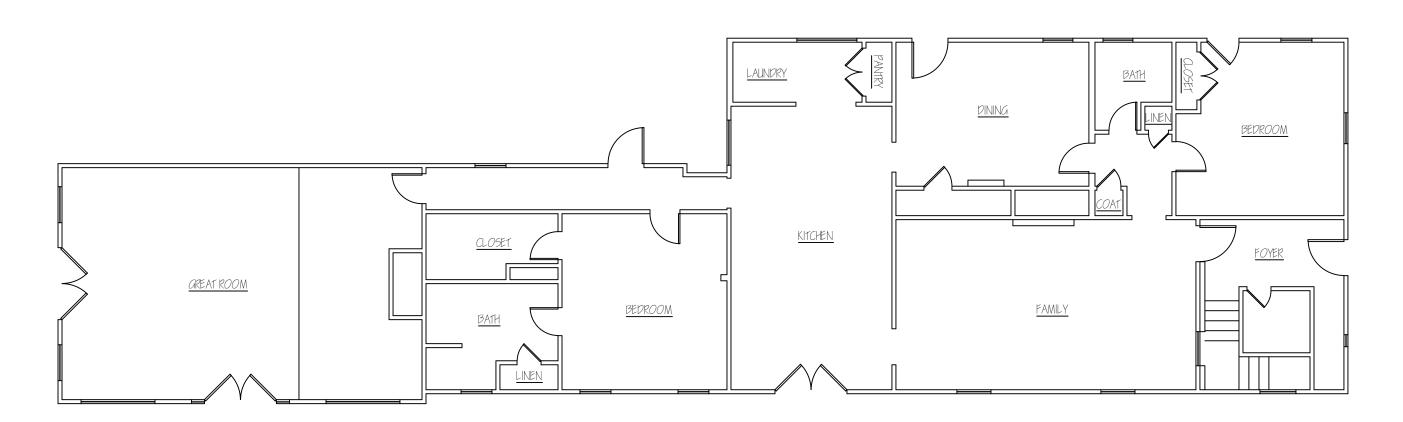
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DEMOLITION SCOPE OF WORK NOTES: I. REMOVE ALL SPECIFIED WALLS DOWN TO FLOOR DECKING. 2. REMOVE AND REPLACE DESIGNATED EXISTING DOORS. 3. REMOVE AND REPLACE DESIGNATED EXISTING WINDOWS.



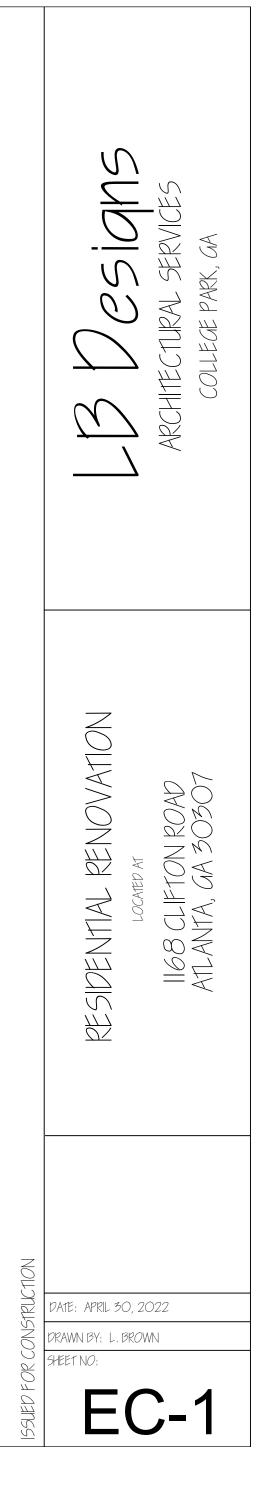


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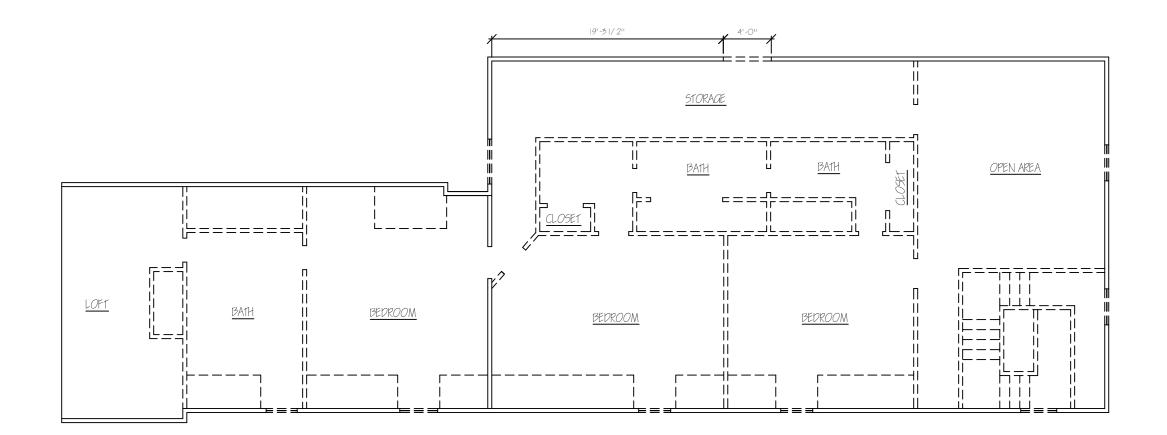
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 WALLS TO BE REMOVED

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 WINDOWS TO BE REMOVED

 >
 DOORS TO BE REPLACED

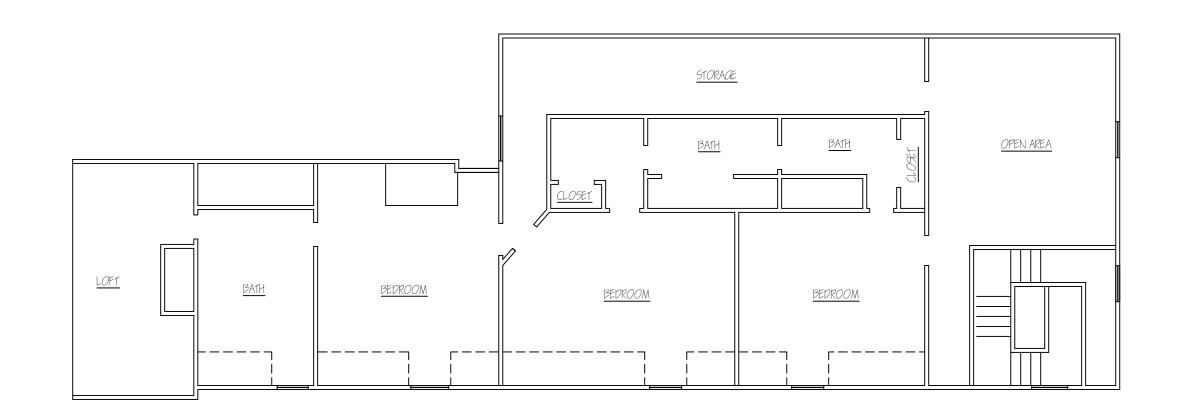






<u>DEMOLITION SCOPE OF WORK NOTES:</u> I. REMOVE ALL SPECIFIED WALLS DOWN TO FLOOR DECKING. 2. REMOVE AND REPLACE DESIGNATED EXISTING DOORS. 3. REMOVE AND REPLACE DESIGNATED EXISTING WINDOWS.





DEMOLITION LEGEND:



Ň ARCHITECTURAL SERVICES CSIDN COLLEGE PARK, GA \mathcal{O} RESIDENTIAL RENOVATION 1168 CLIFTON ROAD ATLANTA, GA 30307 LOCATED AT ISSUED FOR CONSTRUCTION DATE: APRIL 30, 2022 DRAWN BY: L. BROWN SHEET NO: EC-2

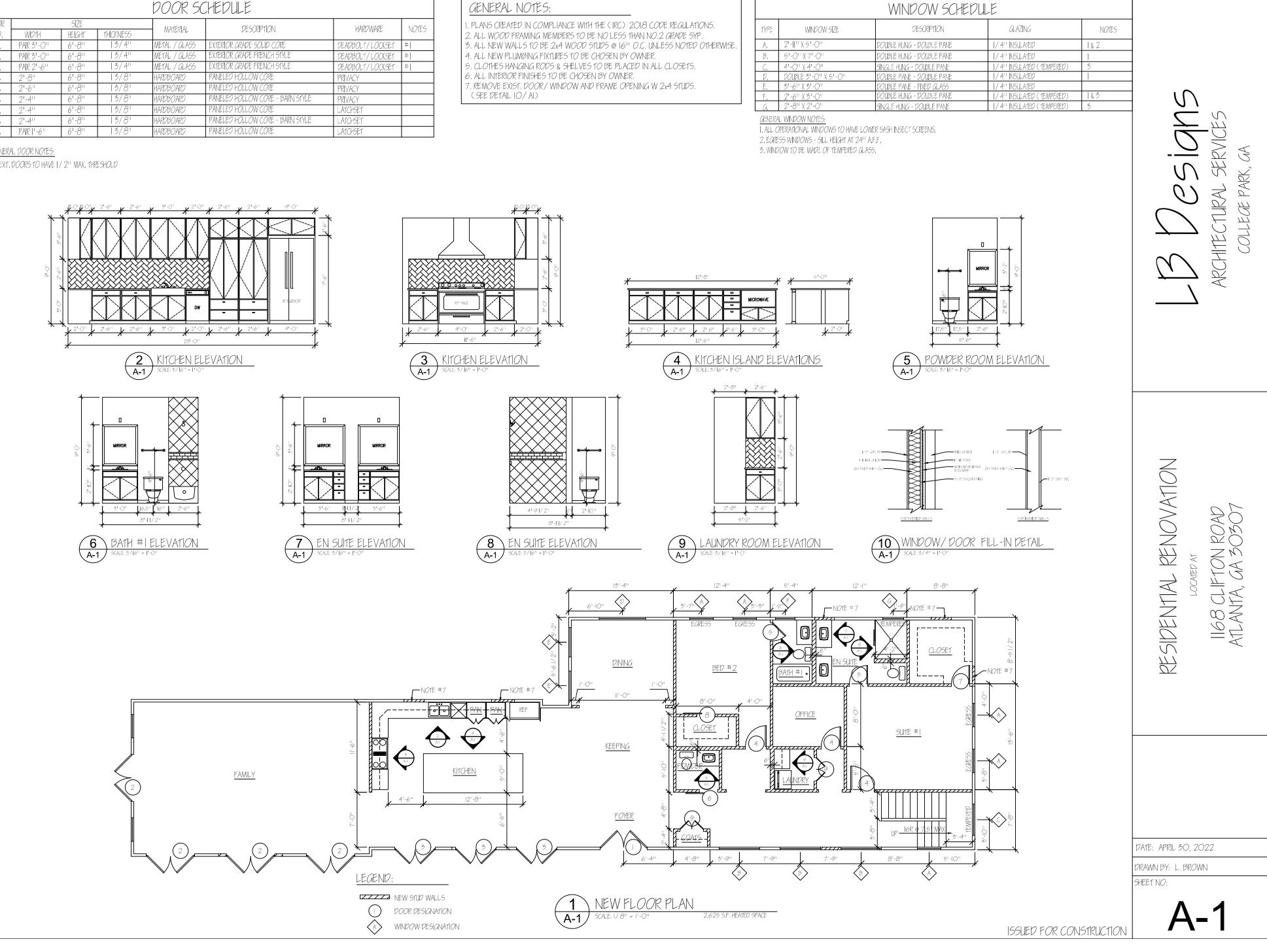
				DOOR S	CHEDULE						
DOOR	SIZE								DESCRIPTION	HARDWARE	NOTES
NO.	WD1H	HEIGHT	1HCKNESS	MATERIAL							
i	PAR 3'-0''	6'-8''	3/4"	METAL / GLASS	EXTERIOR GRADE SOLID CORE	DEADBOLT/LOCKSET	#				
2,	PAIR 3'-0''	6'-8"	3/ 4"	METAL / GLASS	EXTERIOR GRADE FRENCH STYLE	DEADBOLT/LOCKSET	#				
3.	PAIR 2'-6''	6'-8''	3/ 4"	METAL / GLASS	EXTERIOR GRADE FRENCH STYLE	DEADBOLT/LOCKSET	#				
4	2'-8''	6'-8''	3/ 8"	HARDBOARD	PANELED HOLLOW CORE	PRIVACY					
5.	2'-6''	6'-8"	3/ 8"	HARDBOARD	PANELED HOLLOW CORE	PRIVACY					
6.	2'-4"	6'-8"	3/ 8"	HARDBOARD	PANELED HOLLOW CORE - BARN STYLE	PRIVACY					
7.	2'-4"	6'-8"	13/8"	HARDBOARD	PANELED HOLLOW CORE	LATCHSET					
8.	2'-4"	6'-8''	3/8"	HARDBOARD	PANELED HOLLOW CORE - BARN STYLE	LATCHSET					
9.	PAIR '-6''	6'-8"	3/8"	HARDBOARD	PANELED HOLLOW CORE	LATCHSET					

GENERAL NOTES:

- I, PLANS CREATED IN COMPLIANCE WITH THE (IRC) 2018 CODE REGULATIONS.
- 2, ALL WOOD FRAMING MEMBERS TO BE NO LESS THAN NO.2 GRADE SYP.

GENERAL DOOR NOTES:

I, EXT, DOORS TO HAVE I/ 211 MAX, THRESHOLD



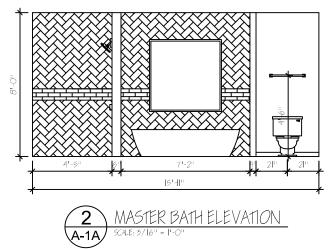
				DOOR S	CHEDULE		
DOOR	SIZE			MAREDIN	DESCRIP110N	HARDWARE	NOREC
NO,	WIDTH	HEIGHT	1HICKNE55	MATERIAL	DEDUKIF HUN	MAKD WAKE	NOTES
,	2'-8''	6'-8"	3/ 8''	HARDBOARD	PANELED HOLLOW CORE	PRIVACY	İ
2,	2'-6''	6'-8"	3/ 8''	HARDBOARD	PANELED HOLLOW CORE - BARN STYLE	PRIVACY	
3,	2'-4''	6'-8"	3/ 8"	HARDBOARD	PANELED HOLLOW CORE	PRIVACY	
4.	2'-6''	6'-8"	3/ 8"	HARDBOARD	PANELED HOLLOW CORE - BARN STYLE	LATCHSET	
5.	2'-0"	6'-8"	3/ 8''	HARDBOARD	PANELED HOLLOW CORE	LATCHSET	
6.	2'-4''	6'-8"	3/ 8"	HARDBOARD	PANELED HOLLOW CORE - BARN STYLE	LATCHSET	
7.	2'-8''	6'-8"	<i>3</i> / 8''	HARDBOARD	PANELED HOLLOW CORE - BARN STYLE	LATCHSET	

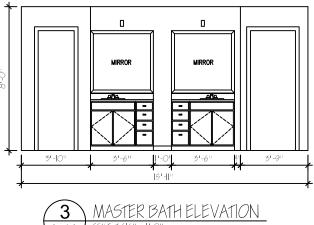
- I, PLANS CREATED IN COMPLIANCE WITH THE (IRC) 2018 CODE REGULATIONS.
- 2, ALL WOOD FRAMING MEMBERS TO BE NO LESS THAN NO.2 GRADE SYP.

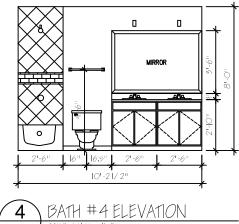
- (SEE DETAIL 6/AI)

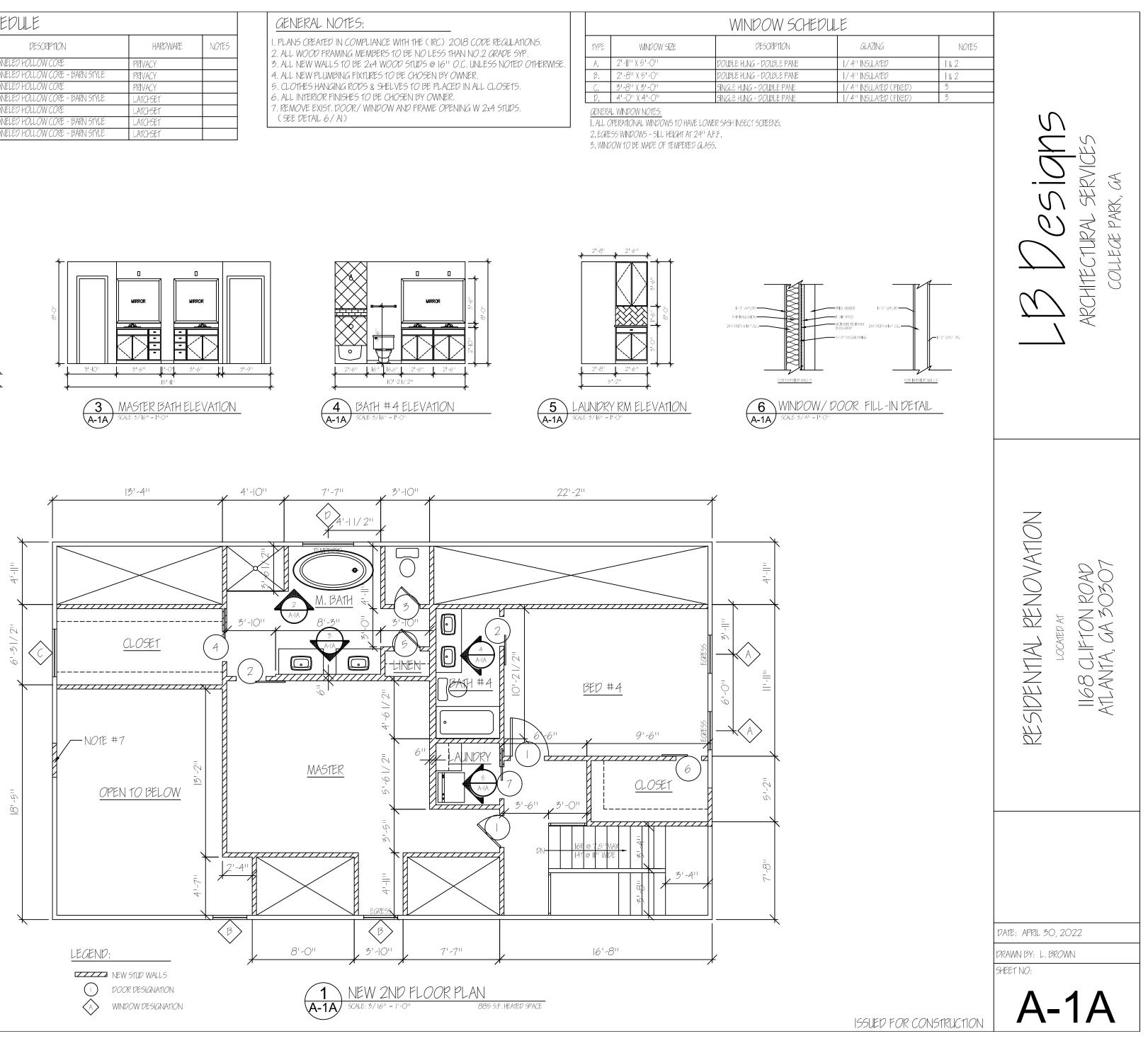
GENERAL DOOR NOTES;

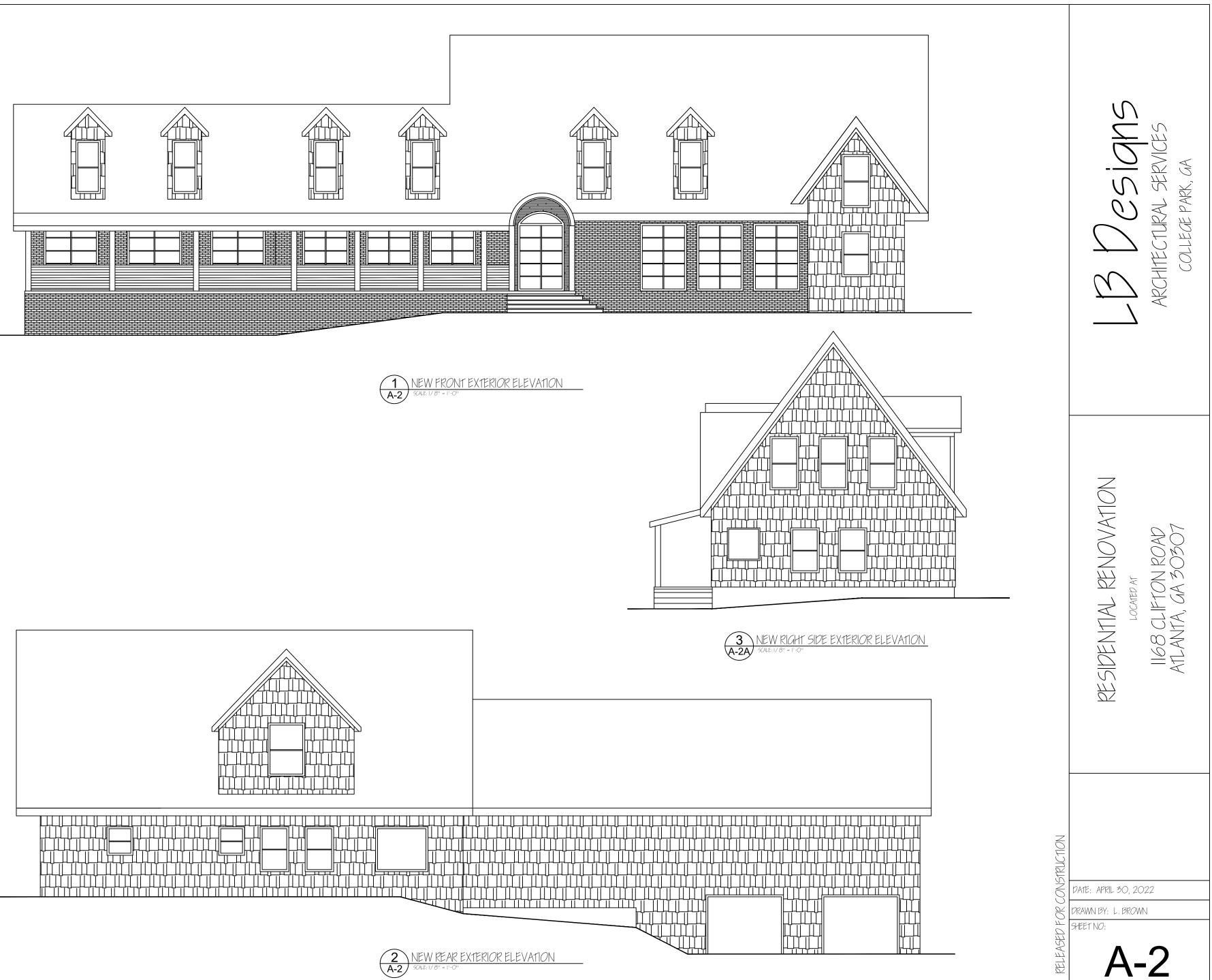
I, EXT, DOORS TO HAVE I/ 211 MAX, THRESHOLD



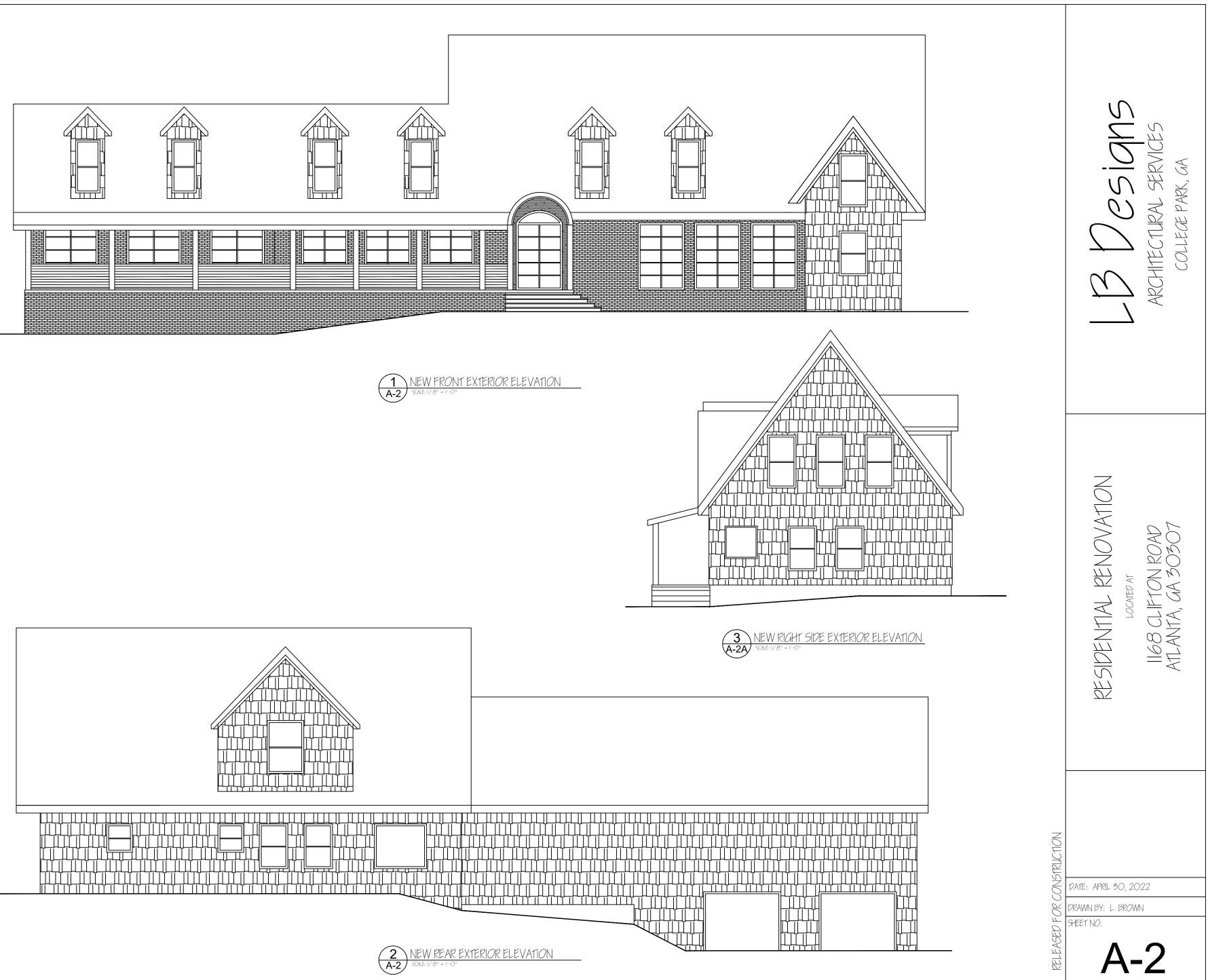


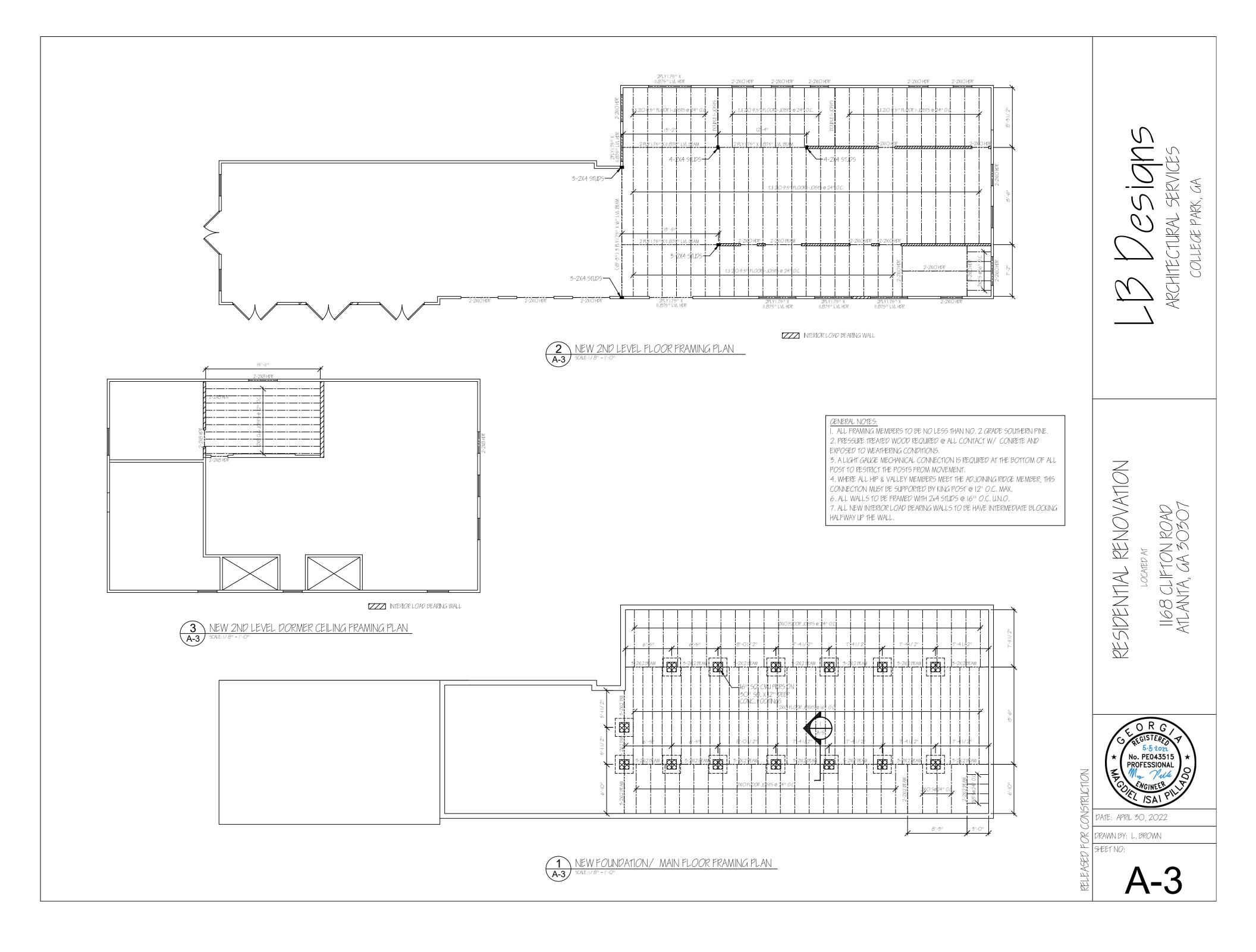


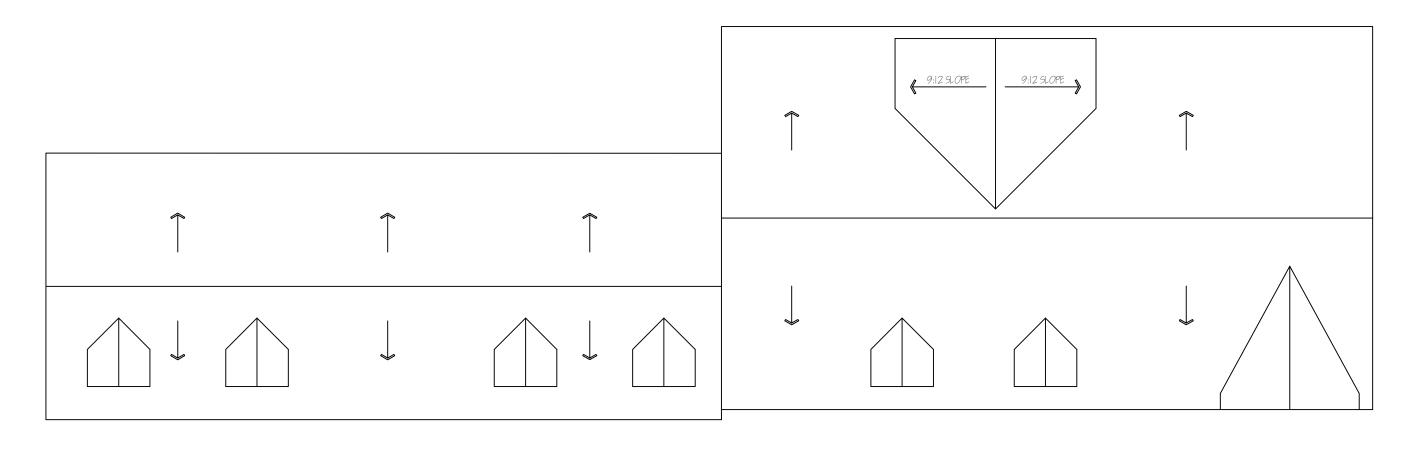


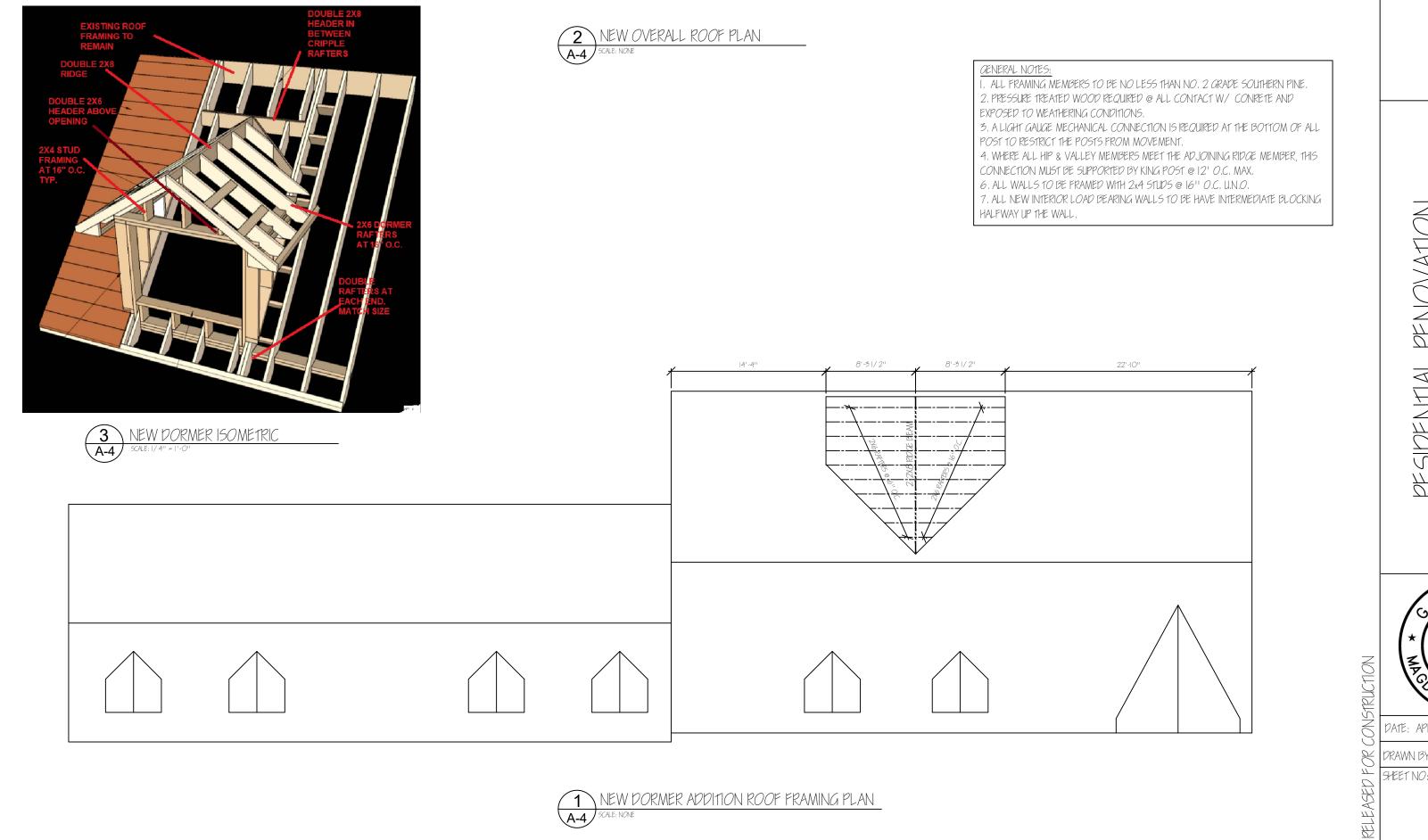






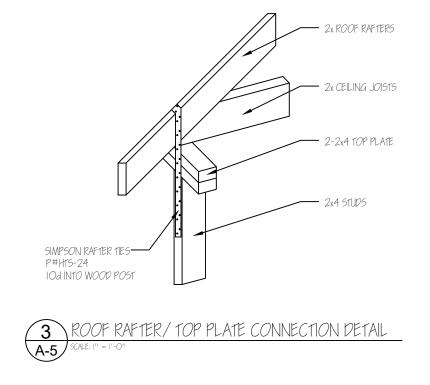


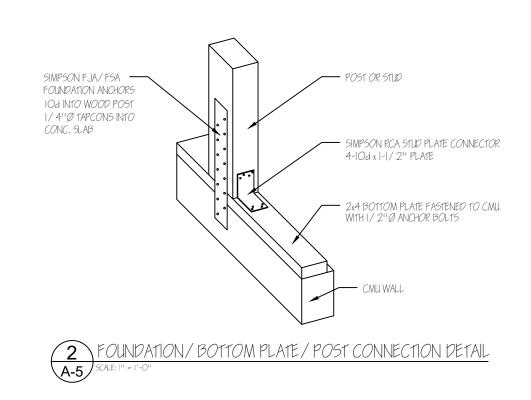


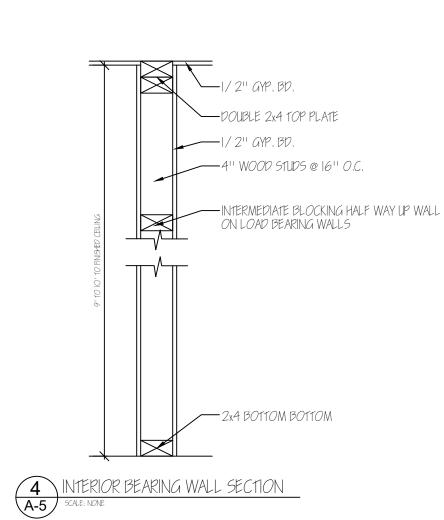


RESIDENTIAL RENOVATION LOCATED AT 1168 CLIFTON ROAD Atlanta, GA 30307 ORG 5-5 2021 No. PE043515 PROFESSIONAL DATE: APRIL 30, 2022 DRAWN BY: L. BROWN SHEET NO: A-4

 \mathcal{O} SERVICES COLLEGE PARK, GA $\frac{0}{5}$ ARCHITECTURAL







GENERAL NOTES:

, ALL FRAMING MEMBERS TO BE NO LESS THAN NO. 2 GRADE SOUTHERN PINE, 2. PRESSURE TREATED WOOD REQUIRED @ ALL CONTACT W/ CONRETE AND

EXPOSED TO WEATHERING CONDITIONS.

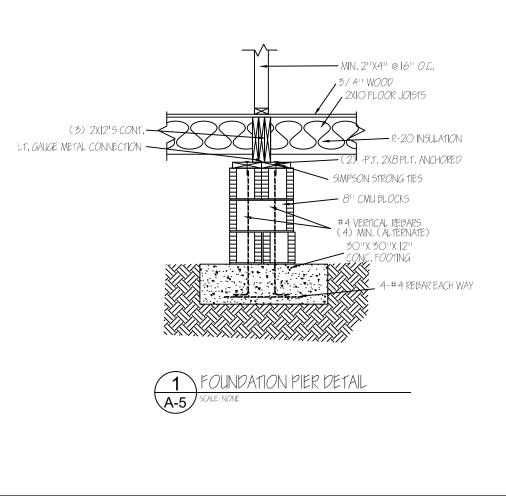
3. A LIGHT GAUGE MECHANICAL CONNECTION IS REQUIRED AT THE BOTTOM OF ALL POST TO RESTRICT THE POSTS FROM MOVEMENT.

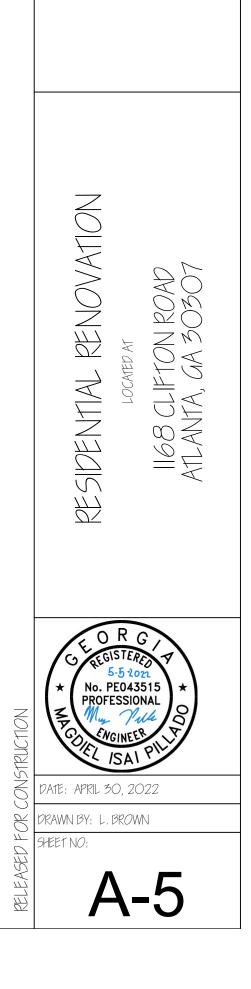
4. WHERE ALL HIP & VALLEY MEMBERS MEET THE ADJOINING RIDGE MEMBER, THIS

CONNECTION MUST BE SUPPORTED BY KING POST @ 12' O.C. MAX. 6. ALL WALLS TO BE FRAMED WITH 2x4 STUDS @ 16" O.C. U.N.O.

7. ALL NEW INTERIOR LOAD BEARING WALLS TO BE HAVE INTERMEDIATE BLOCKING HALFWAY UP THE WALL.



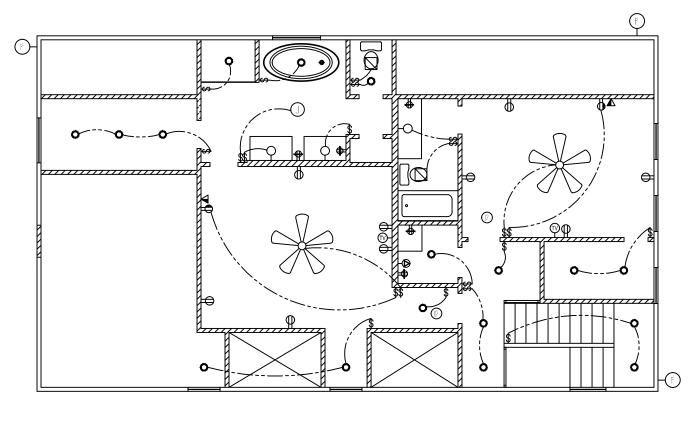




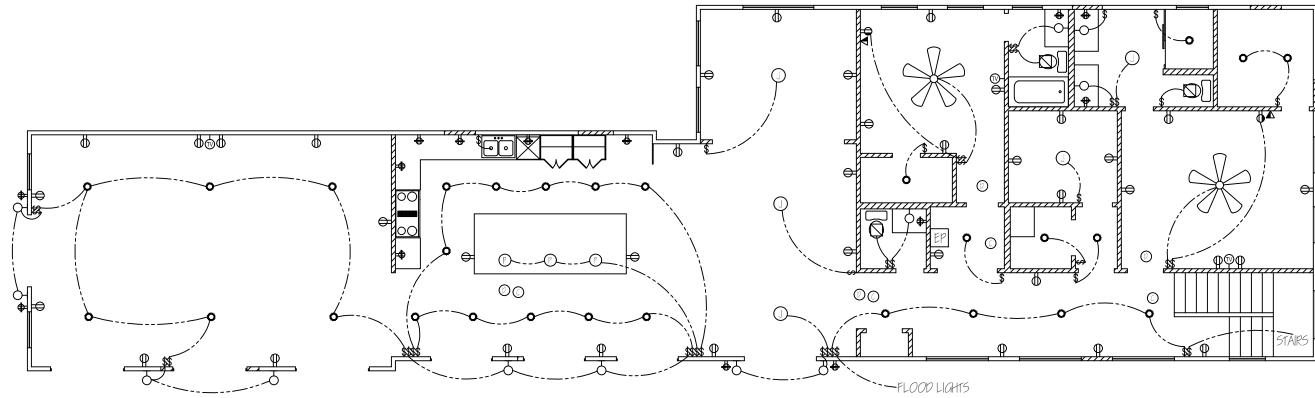
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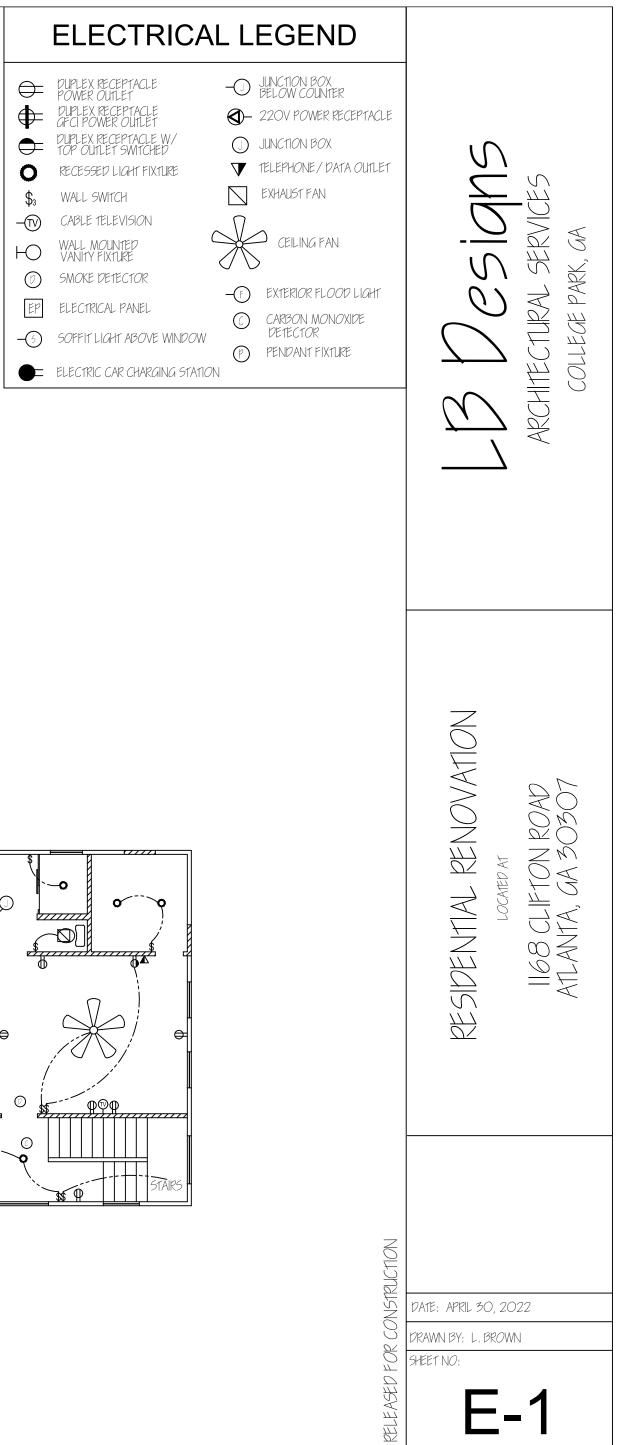
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2 E-1 NEW 2ND FLOOR ELECTRICAL LAYOUT SCALE: 1/8" = 1'-0

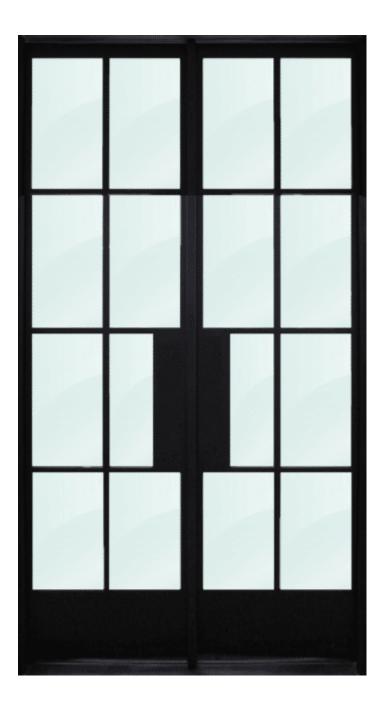


NEW MAIN FLOOR ELECTRICAL LAYOUT (1) (E-1)



Doors:

8-Lite, Slim Steel Double Door with Clear Glass, 59.5" x 80.5" (#8583R/#8584L)



Product DescriptionModel #8-LiteHCO Item #8583-RH, 8584-LHDoor TypeExterior ConfigurationDouble DoorAssembled Width (in.)59.5"Thickness2"

Assembled Depth (in.) 2.25" Gauge 12 gauge Assembled Height (in.) 80.5" Insulation Polyurethane Foam Core Frame Shape Square 1.5" x 2.25" Frame Dimensions Door Finish Black Glass Style Clear Glass Detail Fixed Hand RH & LH Swing Inswing Bore Detail Handle Included Ball Catch 5 Year Iron; 1 Year Glass Warranty



VistaLuxe WD LINE direct sets Pine interior, primed for finishing on the job site 1-1/8" performance divided lites ID: AK614